

Rumus PPTB utk berbagai bentuk saluran

rumus umum PPTB (berlaku utk sembarang profil)

$$\frac{dh}{ds} = i \left\{ \frac{1 - \frac{SQ^2 \cdot P}{iA^3}}{1 - \frac{\alpha Q^2 \cdot B}{gA^3}} \right\} \dots (A)$$

dimana :

dh = selisih dalam air antara 2 potongan saluran

ds = jarak antara 2 potongan saluran

i = kemiringan dasar saluran

 $S = \frac{1}{C^2}$, C adalah koefisien Chezy.

Q = debit saluran

P = kell. basah

A = luas basah

 α = koef. Coriolis

g = gravitasi

B = lebar muka air

$$\text{atau } S = \frac{n^2}{R^{4/3}}$$

n = koef. Manning

h normal (= H), jika :

$$\frac{A^3}{P} = \frac{SQ^2}{i} \dots (I)$$

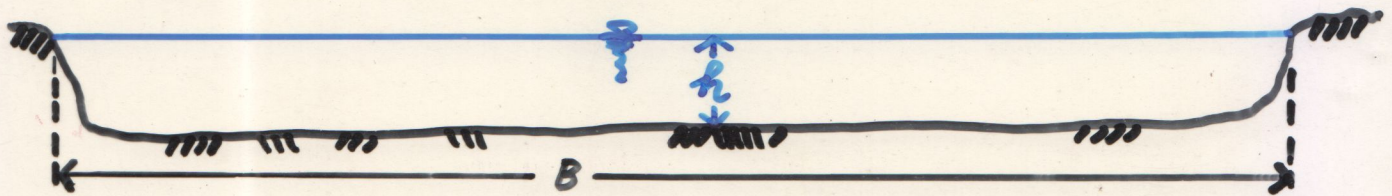
h_{kr} jika :

$$\frac{A^3}{B} = \frac{\alpha Q^2}{g} \dots (II)$$

$$u_{kr}^3 = \frac{gQ}{\alpha B_{kr}} \quad u_{kr}^2 = \frac{gA_{kr}}{\alpha B_{kr}} \dots (III)$$

$$i_{kr} = \frac{Sg}{\alpha} \cdot \frac{P_{kr}}{B_{kr}} \dots (IV)$$

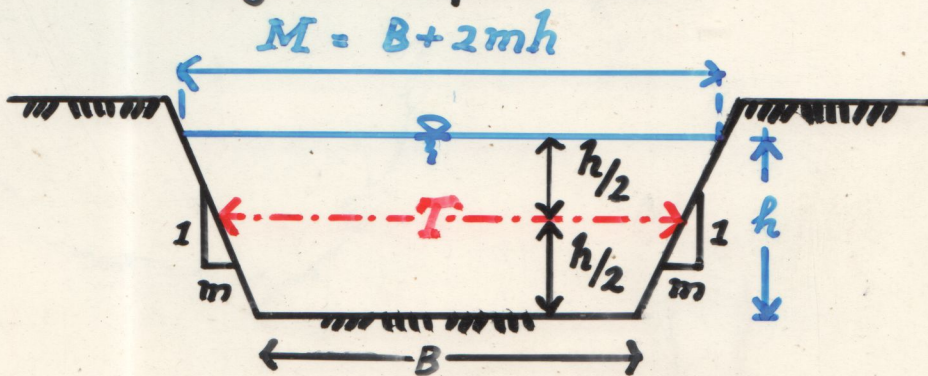
Profil Saluran dg $B = \infty$, $B \gg h$



Debit tiap satuan lebar $q = \frac{Q}{B}$
 $A = B \cdot h$ } $q = \bar{u}h$
 $P = B$

1. $H^3 = \frac{5q^2}{i}$	3. $u_{kr}^3 = \frac{gq}{\alpha}$
2. $h_{kr}^3 = \frac{\alpha q^2}{g}$	4. $i_{kr} = \frac{5g}{\alpha}$

Profil Trapesium.



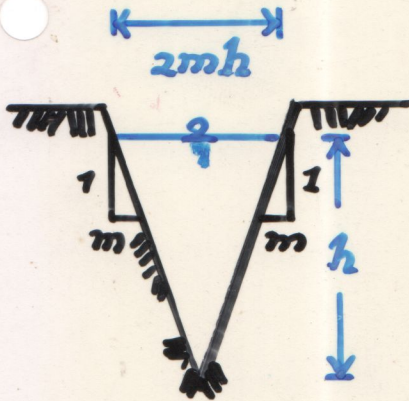
$T = B + mh$
 $A = (B + mh)h = T \cdot h$
 $P = B + 2h \sqrt{1 + m^2}$

(Empat persegi panjang = Trapesium dg $m = 0$)

1. $H^3 = \frac{5Q^2}{i} \cdot \frac{B + 2H\sqrt{1+m^2}}{(B + mH)^3}$	3. $u_{kr}^2 = \frac{g}{\alpha} \cdot \frac{(B + mh_{kr}) \cdot h_{kr}}{B + 2mh_{kr}}$
2. $h_{kr}^3 = \frac{\alpha Q^2}{g} \cdot \frac{B + 2mh_{kr}}{(B + mh_{kr})^3}$	4. $i_{kr} = \frac{5g}{\alpha} \cdot \frac{B + 2h_{kr}\sqrt{1+m^2}}{B + 2mh_{kr}}$

Profil Segitiga.

Profil Δ samakaki \equiv profil trapesium dengan $B = 0$



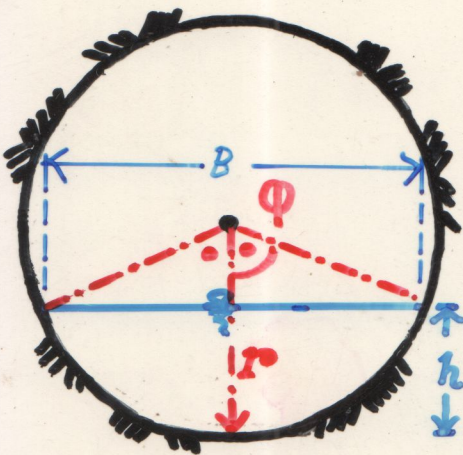
$$1. H^5 = \frac{5Q^2}{i} \cdot \frac{2\sqrt{1+m^2}}{m^3}$$

$$2. h_{kr}^5 = \frac{\alpha Q^2}{g} \cdot \frac{2}{m^2}$$

$$3. u_{kr}^2 = \frac{gh_{kr}}{2\alpha}$$

$$4. i_{kr} = \frac{5g}{\alpha} \cdot \frac{\sqrt{1+m^2}}{m}$$

Profil Lingkaran



ϕ dlm radial radian!

$$P = 2\phi r$$

$$A = r^2 \left(\phi - \frac{1}{2} \sin 2\phi \right)$$

$$B = 2r \sin \phi$$

$$h = r (1 - \cos \phi)$$

1. H jika :

$$\frac{(\phi - \frac{1}{2} \sin 2\phi)^3}{2\phi} = \frac{5Q^2}{ir^5}$$

3.

$$u_{kr}^2 = \frac{g}{\alpha} \cdot \frac{r(\phi_{kr} - \frac{1}{2} \sin 2\phi_{kr})}{2 \sin \phi_{kr}}$$

2. h_{kr} jika :

$$\frac{(\phi_{kr} - \frac{1}{2} \sin 2\phi_{kr})^3}{2 \sin \phi_{kr}} = \frac{\alpha Q^2}{gr_{kr}^5}$$

4.

$$i_{kr} = \frac{5g}{\alpha} \cdot \frac{\phi_{kr}}{\sin \phi_{kr}}$$

Karakteristik Bentuk Aliran.

rumus PPTB utk saluran psg pj lebar ($B = \infty$)

$$1. \quad \frac{dh}{ds} = i \frac{1 - \frac{S q_n^2}{i h^3}}{1 - \frac{\alpha q_n^2}{g h^3}}$$

$$2. \quad H^3 = \frac{S q_n^2}{i}$$

$$3. \quad h_{kr}^3 = \frac{\alpha q_n^2}{g}$$

dari ketiga pers. diatas didapat :

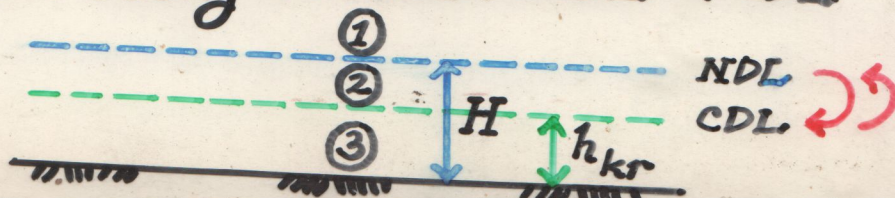
$$\frac{dh}{ds} = i \frac{h^3 - H^3}{h^3 - h_{kr}^3}$$

Bentuk Aliran (flow profile) terlihat dari kurva muka air dari aliran. Secara grs besar terdapat 2 macam bentuk aliran yi :

1. Backwater, jika dalam air h bertambah searah aliran ($dh/ds > 0$)
2. Drawdown, jika dalam air h berkurang searah aliran ($dh/ds < 0$)

Untuk suatu Q tertentu dan keadaan saluran tertentu, NDL dan CDL akan membagi saluran menjadi 3 zone yi

1. Zone 1 : ruang diatas NDL & CDL
2. Zone 2 : ruang diantara NDL & CDL
3. Zone 3 : ruang dibawah NDL & CDL



Ditinjau pers. $\frac{dh}{ds} = i \cdot \frac{h^3 - H^3}{h^3 - h_{kr}^3}$

1). $\frac{dh}{ds} > 0 \rightarrow$ Backwater !

a. $\begin{matrix} h^3 - H^3 > 0 \rightarrow h > H \\ h^3 - h_{kr}^3 > 0 \rightarrow h > h_{kr} \end{matrix} \left. \vphantom{\begin{matrix} h^3 - H^3 \\ h^3 - h_{kr}^3 \end{matrix}} \right\} \text{Zone 1 !}$
 subcritical flow !

b. $\begin{matrix} h^3 - H^3 < 0 \rightarrow h < H \\ h^3 - h_{kr}^3 < 0 \rightarrow h < h_{kr} \end{matrix} \left. \vphantom{\begin{matrix} h^3 - H^3 \\ h^3 - h_{kr}^3 \end{matrix}} \right\} \text{zone 3 !}$
 super critical flow !

2). $\frac{dh}{ds} < 0 \rightarrow$ Drawdown !

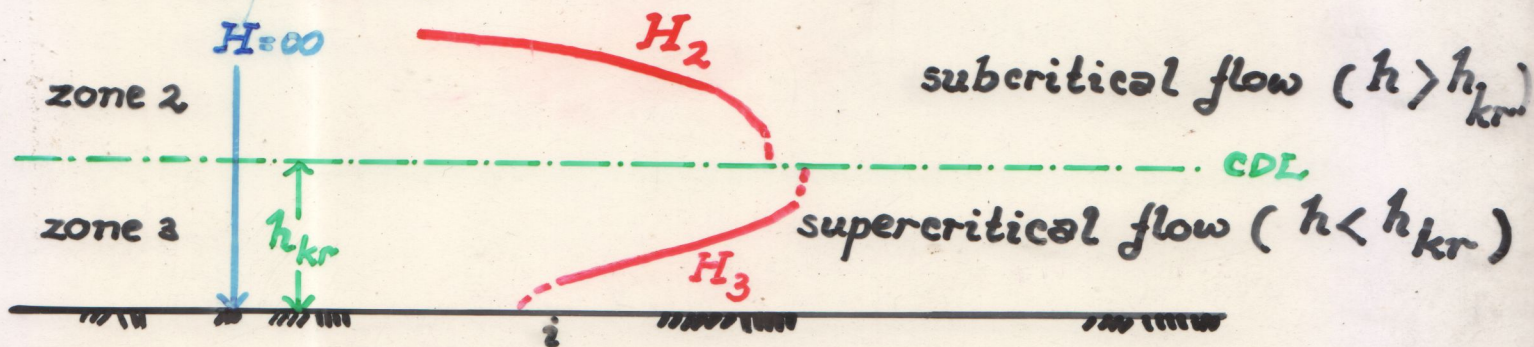
a. $\begin{matrix} h^3 - H^3 > 0 \rightarrow h > H \\ h^3 - h_{kr}^3 < 0 \rightarrow h < h_{kr} \end{matrix} \left. \vphantom{\begin{matrix} h^3 - H^3 \\ h^3 - h_{kr}^3 \end{matrix}} \right\} \text{zone 2 !}$
 supercritical flow !

b. $\begin{matrix} h^3 - H^3 < 0 \rightarrow h < H \\ h^3 - h_{kr}^3 > 0 \rightarrow h > h_{kr} \end{matrix} \left. \vphantom{\begin{matrix} h^3 - H^3 \\ h^3 - h_{kr}^3 \end{matrix}} \right\} \text{zone 2}$
 subcritical flow !

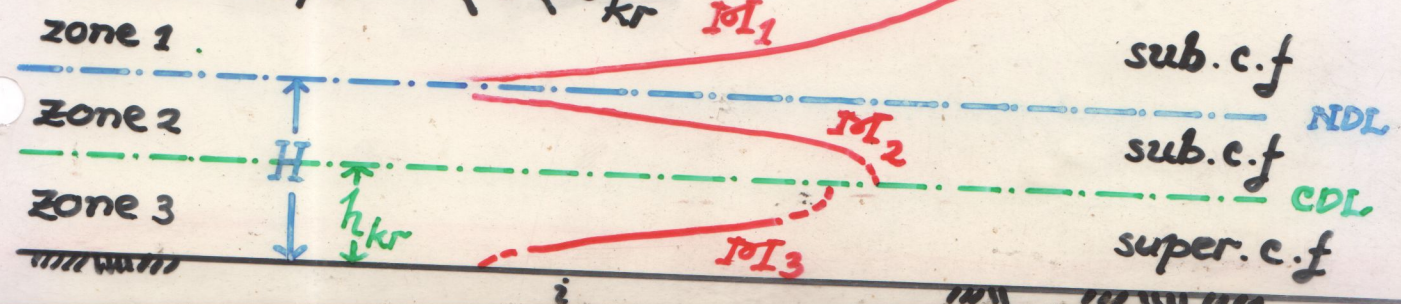
- h_{kr}
 - h
 - H
 - h_{kr}
 - h_{kr}

Tinjauan flow profile untuk bermacam² i saluran :

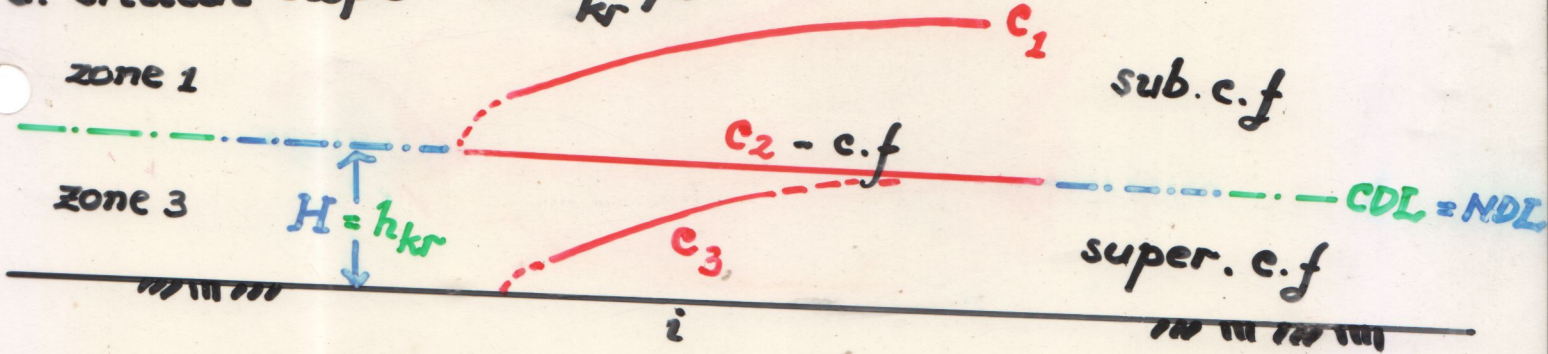
A. Horizontal slope $i=0$



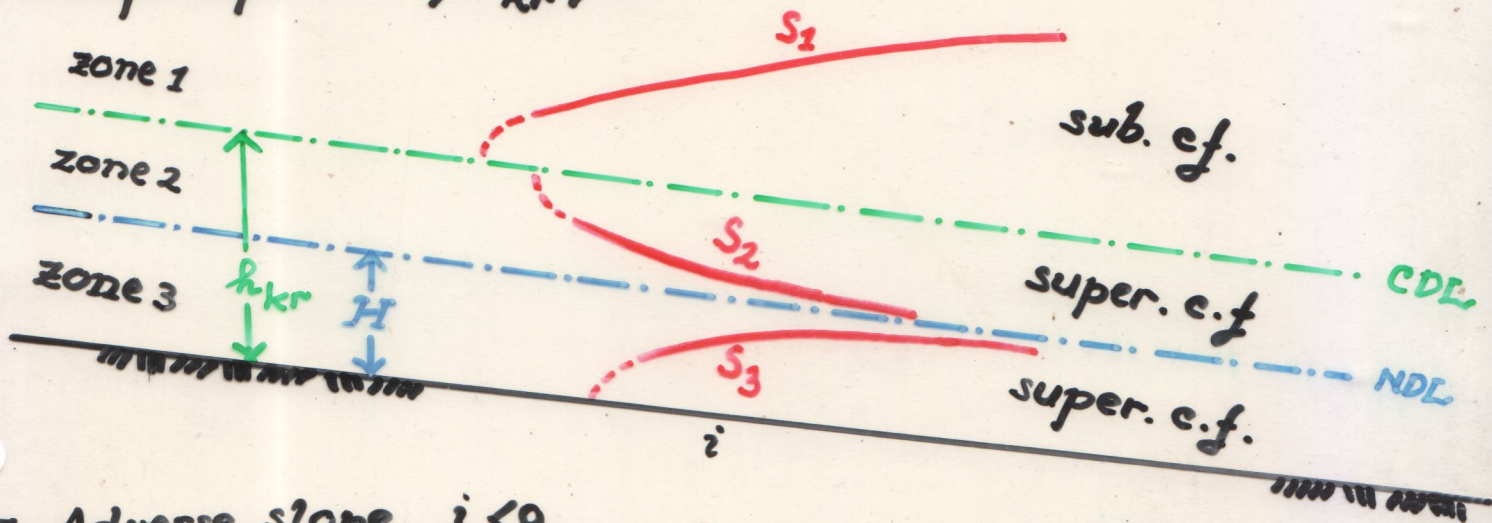
B. Mild slope $0 < i < i_{kr}$



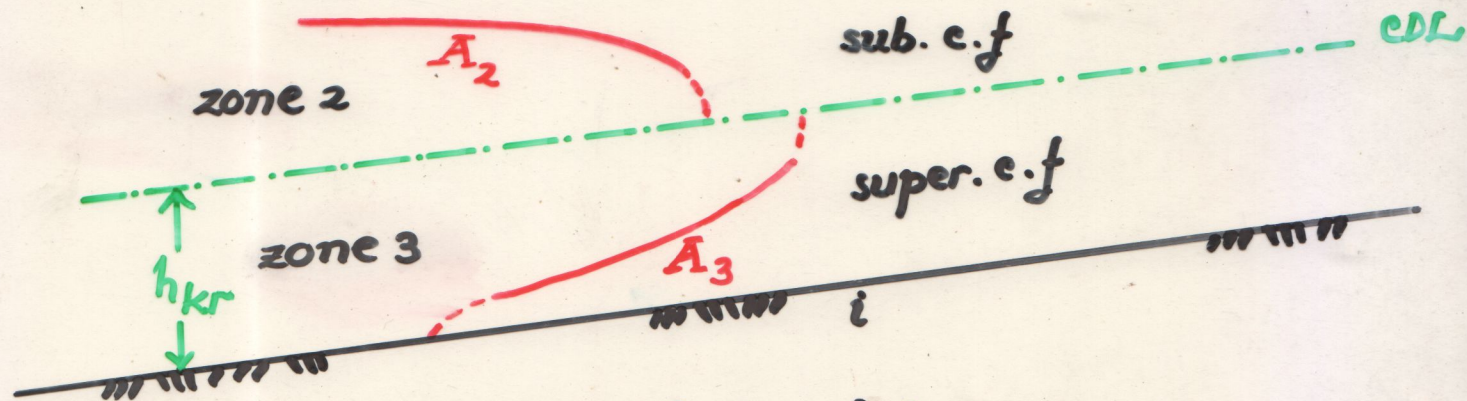
C. Critical Slope $i = i_{kr} > 0$



D. Steep slope $i > i_{kr} > 0$



E. Adverse slope $i < 0$



Tinjauan khusus Adverse slope $i < 0$

$$H^3 = \frac{5q^2}{i} \rightarrow H \text{ negatif} \rightarrow \text{imajiner} \rightarrow \therefore h^3 - H^3 > 0$$

maka

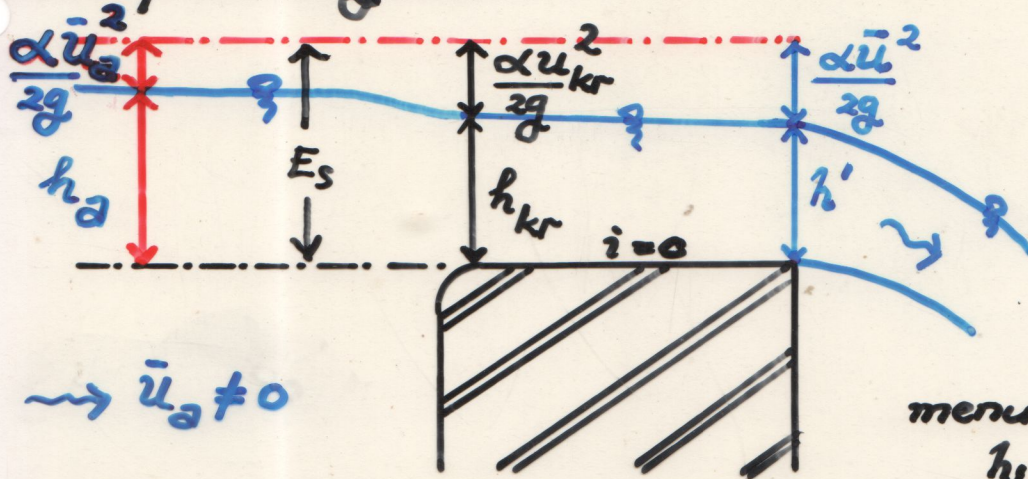
$$\frac{dh}{ds} = i \frac{h^3 - H^3}{h^3 - h_{kr}^3}$$

$\frac{dh}{ds} > 0 \rightarrow$ backwater, jika $h < h_{kr}$ (zone 3)

$\frac{dh}{ds} < 0 \rightarrow$ drawdown, jika $h > h_{kr}$ (zone 2)

Pengaliran melalui peluap.

1. Peluap ambang lebar.



menurut V.T Chow:
 $h_{kr} = 1,4 h'$

Anggapan:

1. ambang datar $\rightarrow i = 0$
2. geseran pd ambang $= 0 \rightarrow h_f = 0$

Pers. umum PPTB:

$$i ds - dh = \frac{P}{A} \cdot \frac{\bar{u}^2}{c^2} \cdot ds + \alpha \frac{d\bar{u}^2}{2g}$$

$$i ds - dh = h_f + \alpha \frac{d\bar{u}^2}{2g}$$

Jadi $dh + \alpha \frac{d\bar{u}^2}{2g} = 0 \dots (1)$

$$E_s = h + \alpha \frac{\bar{u}^2}{2g} \rightarrow \frac{dE_s}{dh} = \frac{dh + \alpha d(\frac{\bar{u}^2}{2g})}{dh} \dots (2)$$

(1) & (2) didapat $\frac{dE_s}{dh} = 0$

Kesimpulan: pada peluap ambang lebar dan datar E_s minimum \rightarrow jadi dalam air diatas ambang adalah h_{kr} .

Jadi ambang lebar akan meluapkan suatu debit tertentu dengan menggunakan energi sekecil mungkin (E_s minimum) d.p.l ambang lebar akan meluapkan Q sebesar mungkin pd suatu harga E_s tertentu.

Prinsip ini dikenal dg nama :

"Prinsip des maximalen durchflusses von Belanger"
(th 1849)

Hitungan Q yg mell. peluap :

Dari bab E_s di depan didapat bahwa pada h_{kr} :

$$\frac{\alpha \bar{u}_{kr}^2}{2g} = \frac{D}{2} \rightarrow \text{dimana } D = \text{hydraulic mean depth}$$

$$\text{Ambang} \rightarrow 4 \text{ psq pj} \rightarrow D = h_{kr}$$

$$E_{s_{kr}} = h_{kr} + \frac{\alpha \bar{u}_{kr}^2}{2g}$$

$$= h_{kr} + \frac{1}{2} h_{kr} \rightarrow h_{kr} = \frac{2}{3} E_s \quad (\text{I})$$

Sedangkan menurut rumus PPTB saluran 4 psq pj :

$$u_{kr}^2 = \frac{gA}{\alpha B} \rightarrow u_{kr}^2 = \frac{g}{\alpha} h_{kr}$$

Debit tiap satuan lebar peluap :

$$q = u_{kr} \cdot h_{kr} \cdot \mu \rightarrow \mu = \text{koefisien peluap.}$$

$$= \mu \sqrt{\frac{g}{\alpha} h_{kr}} \cdot h_{kr}$$

$$\text{Jadi : } q = \mu \sqrt{\frac{g}{\alpha}} \cdot h_{kr}^{3/2} \quad (\text{II})$$

II. Peluap trapesium ambang lebar.

$$\text{Analog diatas : } E_{s_{kr}} = h_{kr} + \frac{D}{2}$$

$$D = \frac{\text{Luas Basah}}{\text{Lebar m.a}} = \frac{T_{kr} \cdot h_{kr}}{M_{kr}}$$

$$E_s = \left(1 + \frac{I_{kr}}{2M_{kr}}\right) h_{kr} \quad (I)$$

Dari PPTB didapat :

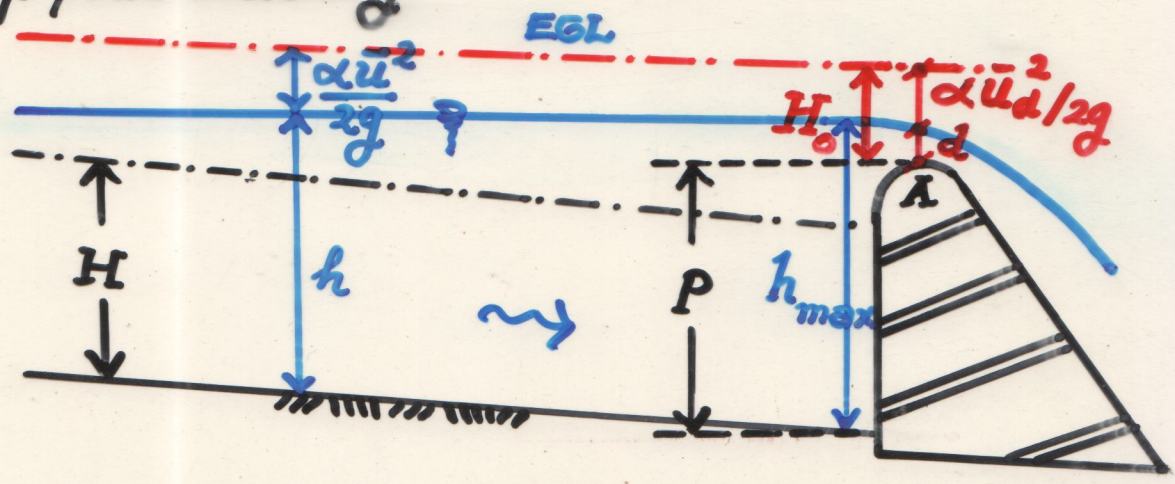
$$u_{kr}^2 = \frac{g}{\alpha} \cdot \frac{A_{kr}}{M_{kr}}$$

Jadi

$$Q = \mu A_{kr} u_{kr} = \mu \cdot I_{kr} h_{kr} \cdot \sqrt{\frac{g}{\alpha} \frac{I_{kr} \cdot h_{kr}}{M_{kr}}}$$

$$Q = \mu h_{kr}^{3/2} \sqrt{\frac{g}{\alpha} \frac{I_{kr}^3}{M_{kr}}} \quad (II)$$

II. Peluap pada bendung.



Pada titik A dianggap kejadiannya sama dg peluap am- bang lebar, datar :

$$h_{kr} = \frac{2}{3} E_{skr}$$

disini jarak antara h_{max} dan titik A adalah pendek shg $h_f = 0 \rightarrow E_{skr} = E_s$ pd h_{max}

Lihat gambar :

$$h_{kr} = \frac{2}{3} E_s \text{ pd } h_{\max}$$

$$d = \frac{2}{3} H_0 \quad (I)$$

Debit tiap satuan lebar peluap :

$$q = \mu \sqrt{\frac{g}{\alpha}} \cdot h_{kr}^{3/2}$$

$$q = \mu \sqrt{\frac{g}{\alpha}} \cdot d^{3/2} \quad (II)$$

Jika P = tinggi bendung, maka

$$P + H_0 = h_{\max} + \alpha \frac{\bar{u}^2}{2g}$$

$$P + H_0 = h_{\max} + \alpha \frac{q^2}{2g h_{\max}^2} \quad (III)$$

$$H_0 = \frac{3}{2} d$$

$$= \frac{3}{2} \left(\frac{q}{\mu} \sqrt{\frac{\alpha}{g}} \right)^{2/3}$$

Jadi pers III menjadi

$$P + \frac{3}{2} \left(\frac{q}{\mu} \right)^{2/3} \left(\frac{\alpha}{g} \right)^{1/3} = h_{\max} + \alpha \frac{q^2}{2g h_{\max}^2} \quad (IV)$$

Tampak bahwa pers terakhir merupakan fungsi h_{\max}^3 jadi h_{\max} dpt dicari dg trial & error dg menggunakan pers (I, II, III) atau pers (IV)

$$\text{Pers IV: } P + \frac{3}{2} \left(\frac{q^2}{\mu^2} \cdot \frac{\alpha}{g} \right)^{1/3} = \frac{2g h_{\max}^3 + \alpha q^2}{2g h_{\max}^2}$$